


|  PRECAST PANELS ON STEEL GIRDERS |                        |          |           |                   |         |         |
|--|------------------------|----------|-----------|-------------------|---------|---------|
| Panel Type   | Maximum Girder Spacing | Slab "T" | Panel "P" | Number of Strands |         |         |
|  |                        |          |           | Flange Width      |         |         |
|  |                        |          |           | 1'-0"             | ≤ 1'-6" | ≤ 2'-0" |
| A1   | 7'-6"                  | 8"       | 3 1/2"    | 15                | 15      | 15      |
| A2   | 8'-0"                  | 8"       | 3 1/2"    | 15                | 15      | 15      |
| A3   | 8'-6"                  | 8"       | 3 1/2"    | 17                | 16      | 16      |
| A4   | 9'-0"                  | 8"       | 3 1/2"    | 19                | 17      | 17      |
| A  | 9'-6"                  | 8"       | 3 1/2"    | 21                | 19      | 18      |
| B  | 10'-0"                 | 8 1/2"   | 3 1/2"    | 22                | 21      | 19      |
| C  | 10'-6"                 | 9"       | 3 1/2"    | 24                | 22      | 20      |
| D  | 11'-0"                 | 9 1/2"   | 3 1/2"    | 27                | 24      | 22      |
| E  | 11'-6"                 | 10"      | 3 1/2"    | 30                | 27      | 25      |
| F  | 12'-0"                 | 10 1/2"  | 3 1/2"    | 33                | 30      | 28      |

**NOTES:**

1. Precast Concrete Deck Panels shall be fabricated in accordance with Section 535 of the Standard Specifications.
2. The contractor shall submit working drawings showing the exact layout of panel types and sizes.
3. Refer to the Design Drawings for structures with curved beams or angled splices.
4. Joints at expansion piers shall be treated similarly to the abutment joint details.
5. Panel widths of less than 8'-0" may be used. Provide strands in the ratio of the smaller panel width to 8'-0", multiplied by the number of strands given in the table, rounding up to the next even number of strands. The minimum panel width is 3'-0"
6. Prestressing strands shall be 3/8-in. diameter Grade 270 seven - wire low relaxation strands conforming to the requirements of ASTM A 416. Initial tension shall be 17.2 kips per strand.

(Continued)

NOTES:

1. Steel for diaphragms, crossframes, connection plates, gussets and stiffeners shall be as designated on the Design Drawings.

△ 2. All welds for diaphragms, crossframes, connection plates, gussets and stiffeners shall terminate  $\frac{5}{8}$ "  $\pm$   $\frac{1}{8}$ " from the ends of the plates.

3. Bolt holes shall be  $\frac{15}{16}$ ". The minimum edge distance shall be  $1\frac{1}{2}$ " unless otherwise shown on the Design Drawings. Oversized holes may be used with the permission of the Resident.

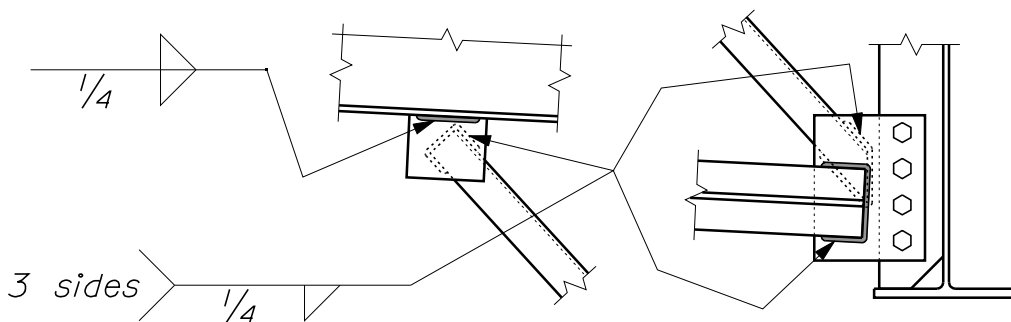
4. Connection plates and gussets shall be  $\frac{3}{8}$ " minimum thickness. Connection plates shall be 7" minimum width. The plate thickness for stiffeners and bent connection plates shall be as shown on the Design Drawings.

5. Connection plates shall be full web depth except that connection plates shall be connected to flanges in tension and stress reversal using the "Tension Flange Connection" detail.

△ 6. Bearing stiffeners shall be mill - to - bear on the bottom flange and tight fit to the top flange. Except at abutments, bearing stiffeners used as connection plates shall be connected to flanges in tension and stress reversal using the "Tension Flange Connection" detail.

7. Intermediate stiffeners not intended to carry concentrated loads shall be tight fit to both flanges. Intermediate stiffeners used as connection plates shall be detailed as connection plates.

△ 8. Stiffeners not used as connection plates shall be welded to the web only. Connection plates and stiffeners used as connection plates shall be welded to the web and flanges (or "Tension Flange Connection" detail) on both sides of the plates.



~ TYPICAL WELD DETAILS ~